

## Review of Response to Random Loads

- We require new mathematical tools to describe “random” loads and responses of structures
- Random loads vary in:
  - Magnitude
  - Time
  - Spatially
- We use:
  - Probability Distributions
  - Power Spectra and Auto-correlations
  - Cross-Spectra and Cross-correlations

### Stationary Random Process:

#### Strict Stationarity

- All statistical properties are invariant with time

#### Weak Stationarity

- Mean  $\mu_x$  and autocorrelation  $\rho_x(\tau)$  are invariant with time

#### Ergodic Process

- Stationary process where the statistical properties of one sample are identical to the ensemble statistics

- There are fully developed mathematical methods available for Ergodic Processes and thus there are enormous advantages if a process can be regarded as stationary. This includes processes that can be described as having *weak stationarity* or *local stationarity*

### Auto-correlation

- $R_x(\tau) = \lim_{T \rightarrow \infty} \frac{1}{T} \int_0^T (x(t) - \bar{x})(x(t - \tau) - \bar{x}) dt$
- $R_x(\tau) = R_x(-\tau)$  (an “even” function)
- $R_x(\tau = 0) = \sigma_x^2 = \lim_{T \rightarrow \infty} \int_0^T (x(t) - \bar{x})^2 dt$
- $\frac{dR(\tau = 0)}{d\tau} = 0$
- $R(\tau = \infty) = 0$
- If  $x(t) = y_1(t) + y_2(t) + y_3(t) + \dots + y_n(t)$  where the  $y$ 's are independent, then  $R_x(\tau) = R_{y_1}(\tau) + R_{y_2}(\tau) + \dots + R_{y_n}(\tau)$  and since  $R_x(\tau = 0) = \sigma_x^2$ , then  $\sigma_x^2(\tau) = \sigma_{y_1}^2 + \sigma_{y_2}^2 + \dots + \sigma_{y_n}^2$

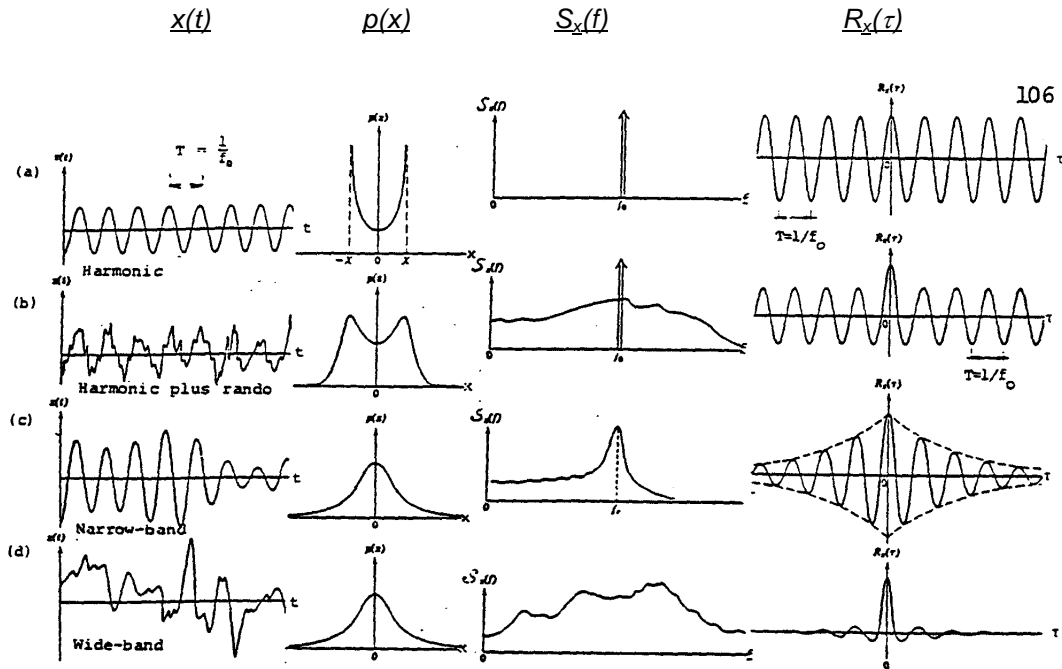
Power Spectrum

- Two-sided spectrum  $G(f) = \int_{-\infty}^{\infty} R(\tau)e^{-i2\pi f\tau} d\tau$
- One-sided spectrum  $S(f)$  defined for  $f \geq 0$

$S(f) = 2G(f)$  and recalling that  $R(\tau) = R(-\tau)$

$$\left. \begin{aligned} S(f) &= 4 \int_0^{\infty} R(\tau) \cos(2\pi f\tau) d\tau \\ R(\tau) &= \int_0^{\infty} S(f) \cos(2\pi f\tau) df \end{aligned} \right\} \text{This is a "Cosine Fourier Transform Pair"}$$

- The area under the power spectrum equals the variance,  $\sigma^2$
- Dimensions of power spectrum are (dimension of  $x^2$ )/frequency, for example if "x" is displacement in metres, then the units of the power spectrum are:  $m^2 / \text{Hz}$
- Power spectra are often normalized by the variance,  $\sigma^2$ , so the area is 1.0
- Examples of Random Processes:



Power Spectra

We defined a Power Spectral Density Function:

$$G(f) = \int_{-\infty}^{\infty} R(\tau) e^{-i2\pi f\tau} d\tau$$

$G(f)$  is a symmetric function about  $f = 0$ . Since  $f < 0$  have little physical meaning, one defines a one-sided power spectrum as  $S(f)$  for  $f \geq 0$ .

To conserve equality of variance;

$$\begin{aligned} S(f) &= 2G(f) = 2 \int_{-\infty}^{\infty} R(\tau) e^{-i2\pi f\tau} d\tau \\ &= 2 \int_{-\infty}^{\infty} R(\tau) \cos 2\pi f\tau d\tau + i2 \int_{-\infty}^{\infty} R(\tau) \sin 2\pi f\tau d\tau \\ &= 0 \text{ Integral of product of even \& odd functions} \\ S(f) &= 4 \int_0^{\infty} R(\tau) \cos 2\pi f\tau d\tau \dots \text{ the one-sided SDF, a Fourier Transform} \end{aligned}$$

Properties of Power Spectra

- Relationship between the Probability Density Function (PDF) and the Spectral Density Function (SDF):

If  $x(t)$  is Gaussian;

$$p_x(x) = \frac{1}{\sqrt{2\pi}\sigma_x} e^{-\frac{(x-\bar{x})^2}{2\sigma_x^2}}$$

where

$$\sigma_x^2 = \int_0^{\infty} S_x(f) df \quad (8.18)$$

When  $y(t) = ax(t) \dots S_y(f) = a^2 S_x(f)$  and  $\sigma_y^2 = a^2 \sigma_x^2$

e.g. if  $p(t) = -m\ddot{y}_g(t)$  then  $S_p(f) = m^2 S_{\ddot{y}_g}(f)$

If  $x(t)$  is a time-varying quantity, then

$p_x(x)$  ... amplitude domain  
 $S_x(f)$  ... frequency domain  
 $R_x(\tau)$  ... correlation with itself at time lag  $\tau$  (see page 106)

How do we describe the time variation of a periodic function?

using a Fourier Series: provides information on variation in frequency at  $\omega$ ,  $2\omega$ ,  $3\omega$  etc. (at discrete harmonics)

How do we describe a Random Variable in Time?

using a Fourier Integral: provides information on variation at all values of  $\omega$

$$\left. \begin{aligned} x(t) &= \frac{1}{2\pi} \int_{-\infty}^{\infty} A(\omega) e^{i\omega t} d\omega \\ A(\omega) &= \int_{-\infty}^{\infty} x(t) e^{-i\omega t} dt \end{aligned} \right\} \text{a Fourier Transform Pair}$$

Response of Structures to Random Loads

If  $P(t) = P_o e^{i\omega t} = P_o e^{i2\pi f t}$  (a single harmonic), then the response of the structure becomes:

$$y(t) = \frac{P_o}{K} H(f) e^{i2\pi f t}$$

The Complex Mechanical Admittance:  $H(f) = \frac{1}{1 - \left(\frac{f}{f_o}\right)^2 + i2D\frac{f}{f_o}}$  (see pg 29b)

If the load consists of many harmonics;

$$\begin{aligned} y(t) &= \sum_{-\infty}^{\infty} \frac{1}{k} c_r H(f_r) e^{ir2\pi f_r t} \\ \overline{y^2} &= \sigma_y^2 = \frac{1}{k^2} \sum_{-\infty}^{\infty} |c_r|^2 |H(f_r)|^2 \end{aligned}$$

If we represent a random load by a periodic function with  $T \rightarrow \infty$  (i.e it never repeats itself)

As  $1/T \rightarrow df$ ,  $rf_1 = f_r \rightarrow f$ , and  $\sum_0^{\infty} \rightarrow \int_0^{\infty}$

$$\text{So, } \overline{y^2} = \int_0^{\infty} S_y(f) df = \int_0^{\infty} S_p(f) |H(f)|^2 df$$

the Modulus of the Complex Mechanical Admittance

$$|H(f)|^2 = \varepsilon^2 = \frac{1}{\left[1 - (f/f_o)^2\right]^2 + 4D^2(f/f_o)^2} \quad (\text{see pg 111})$$

Variance of Response to Random Load  $P(t)$

$$\overline{y^2} = \int_0^{\infty} S_y(f) df$$

$$= \frac{1}{K^2} \int_0^{\infty} S_p(f) |H(f)|^2 df$$

← Difficult to evaluate in closed form

A conservative approximation is:

$$\overline{y^2} \cong \frac{1}{k^2} \int_0^{\infty} S_p(f) df + \frac{1}{k^2} \int_0^{\infty} S_p(f_o) |H(f)|^2 df$$

$$= \frac{1}{k^2} \left[ \sigma_p^2 + \frac{1}{k^2} S_p(f_o) \frac{\pi f_o}{4D} \right]$$

$$\sigma_y^2 = \sigma_{yB}^2 + \sigma_{yR}^2$$

↑ Background

← Resonant

**NOTE:**

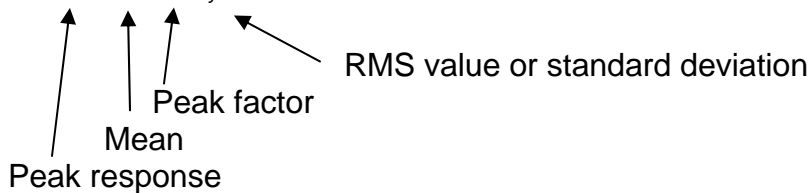
$$\sigma_{yR}^2 \cong \frac{\pi f_o}{4 D} S_p(f_o)$$

$$\sigma_{yR} \cong \sqrt{\frac{\pi f_o}{4 D} S_p(f_o)} \propto \frac{1}{\sqrt{D}} \quad \leftarrow \text{random}$$

$$\propto \frac{1}{D} \quad \leftarrow \text{harmonic}$$

Peak Value of a Random Process (pg 114)

$$\hat{y} = \bar{y} + g\sigma_y \quad (\text{see fig 8.15})$$



$$g = \sqrt{2 \log_e \nu T} + \frac{0.5772}{\sqrt{2 \log_e \nu T}}$$

$$V = \sqrt{\frac{\int_0^\infty f^2 S(f) df}{\int_0^\infty S(f) df}} \approx f_o \text{ for a narrow-band process}$$

The cycling rate, or apparent frequency

Response to Gusting Wind

Point Structure is one where  $\sqrt{A} \ll \lambda(f)$

$$F_D(t) = \frac{1}{2} \rho C_D A V^2(t) = \frac{1}{2} \rho C_D A [\bar{V} + v(t)]^2 \text{ where } \bar{V} \gg v(t) \text{ so,}$$

$$F_D(t) = \frac{1}{2} \rho C_D A \bar{V}^2 + \rho C_D A \bar{V} v(t) \text{ a linearization of the response}$$

mean  $\nearrow$   $\nwarrow$  time varying

$$S_D(f) = \frac{4 \bar{F}_D^2}{\bar{V}^2} S_v(f)$$

BUT Real structures have  $\sqrt{A} \approx \lambda(f)$

Therefore the instantaneous wind induced pressures are not fully correlated over the frontal area of the body, so:

$$S_F = \frac{4 \bar{F}_D^2}{\bar{V}} \left| \chi \left( \frac{fL}{\bar{V}} \right) \right|^2 S_v(f)$$

$\nwarrow$  The Aerodynamic Admittance

$$\left| \chi \left( \frac{\sqrt{A}}{\lambda_f} \right) \right|^2 \rightarrow 1 \text{ as } \frac{\sqrt{A}}{\lambda_f} \rightarrow 0 \text{ and } \left| \chi \left( \frac{\sqrt{A}}{\lambda_f} \right) \right|^2 \rightarrow 0 \text{ as } \frac{\sqrt{A}}{\lambda_f} \rightarrow \infty$$

The National Building Code of Canada (NBCC)

Uses a Gust Factor approach

$$\hat{y} = C_g \bar{y}$$

peak  $\nwarrow$   $\nwarrow$  mean  
gust effect factor

The NBCC contains a procedure for the evaluation of  $C_g$

For:

- i) simple shape buildings
- ii) 1<sup>st</sup> sway mode
- iii) mass distribution must be constant with height
- iv) only drag is considered (lift or crosswind response is not predicted)